PE Naval Architecture & Marine Engineering Reference Handbook (2022)

ERRATA SHEET



1.2 Properties of Materials

1.2.3 Properties of Water at Standard Conditions

Original:

Specific heat, $c_{\rm p}$: In I-P units: $\frac{1\,{\rm Btu}}{{\rm lb}\text{-}{\rm °F}}$ at 68°F In SI units: $\frac{4.180\,{\rm kJ}}{{\rm kg}\cdot{\rm K}}$ at 20°C

Density at standard conditions: $\frac{1,000 \text{ kg}}{\text{m}^3} = \frac{62.4 \text{ lbm}}{\text{ft}^3}$

Specific weight at standard conditions: $\frac{9,810 \text{ N}}{\text{m}^3} = \frac{9,810 \text{ kg}}{\text{m}^2 \cdot \text{s}^2} = \frac{62.4 \text{ lbf}}{\text{ft}^3}$

Updated:

Specific heat, $c_{\rm p}$: In I-P units: $\frac{1\,{\rm Btu}}{{\rm Ib}\text{-}^{\circ}{\rm F}}$ at $68^{\circ}{\rm F}$ In SI units: $\frac{4.180\,{\rm kJ}}{{\rm kg} \cdot {\rm K}}$ at $20^{\circ}{\rm C}$

Density at standard conditions: $\frac{1,000 \text{ kg}}{\text{m}^3} = \frac{62.4 \text{ lbm}}{\text{ft}^3} \quad \text{(fresh water)}$

 $\frac{1026 \text{ kg}}{\text{m}^3} = \frac{64.0 \text{ lbm}}{\text{ft}^3} \text{ (sea water)}$

5

Specific weight at standard conditions: $\frac{9,810 \text{ N}}{\text{m}^3} = \frac{9,810 \text{ kg}}{\text{m}^2 \cdot \text{s}^2} = \frac{62.4 \text{ lbf}}{\text{ft}^3}$

Schedule 80 Steel Pipe

Nominal Pipe Size	Wall Thickness	Inside Diameter	Flow Area	Pipe Wt. per L.F.	Gallons per L.F.	Water Wt. per L.F.	Total Wt. per L.F.	Moment of Inertia	Section Modulus	Radius of Gyration
in.	in.	in.	in ²	lb		lb	lb/	in ⁴	in ³	in.
0.5	0.147	0.546	0.234	1.09	0.012	0.10	1/19	0.020	0.048	0.250
0.75	0.154	0.742	0.432	1.47	0.022	0.19	/1.66	0.045	0.085	0.321
1	0.179	0.957	0.719	2.17	0.044	0.37	2.54	0.106	0.161	0.407
1.25	0.191	1.278	1.282	3.00	0.067	0.56	3.56	0.242	0.291	0.524
1.5	0.200	1.500	1.766	3.63	0.106	0.88	4.51	0.391	0.412	0.605
2	0.218	1.939	2.951	5.02	0.174	1.46	6.48	0.868	0.731	0.766
2.5	0.276	2.323	4.236	7.66	0.220	1.84	9.50	1.920	1.340	0.924
3	0.300	2.900	6.602	10.25	0.383	3.20	13.45	3.890	2.230	1.140
4	0.337	3.826	11.491	14.98	0.660	5.51	20.49	9.610	4.270	1.480
5	0.375	4.813	18.185	20.78	0.945	7.89	28.67	20.700	7.430	1.840
				/						
6	0.432	5.761	26.053	28.57	1.499	12.51	41.08	40.500	12.200	2.190
8	0.500	7.625	45.640	43.39	2. 5 98	21.69	65.08	106.000	24.500	2.880
10	0.593	9.564	71.804	54.74	4.085	34.10	88.84	212.000	39.400	3.630
12	0.687	11.376	101.590	65.42	5.810	48.50	113.92	362.000	56.700	4.330

Double Extra Strong XX Steel Pipe

Nominal Pipe Size	Wall Thickness	Inside Diameter	Flow Area	Pipe Wt. per L.F.	Gallons per L.F.	Water Wt. per L.F.	Total Wt. per L.F.	Moment of Inertia	Section Modulus	Radius of Gyration
in.	in.	in.	in ²	lb		lb	lb	in ⁴	in ³	in.
2	0.436	1.503	1.773	9.03	0.174	1.46	10.49	1.310	1.100	0.703
2.5	0.552	1.771	2.462	13.69	0.128	1.07	14.76	2.870	2.000	0.844
3	0.600	2.300	4.153	18.58	0.383	3.20	21.78	5.990	3.420	1.050
4	0.674	3.152	7.799	27.54	0.660	5 5	33.05	15.300	6.790	1.370
5	0.750	4.063	12.959	38.55	0.674	5.62	44.17	33.600	12.100	1.720
6	0.864	4.897	18.825	53.16	1.499	12.51	65,67	66.300	20.000	2.060
8	0.875	6.875	37.104	72.42	2.598	21.69	94.11	162.000	37.600	2.760

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Updated:

Schedule 80 Steel Pipe

Nominal Pipe Size	Wall Thickness	Inside Diameter	Flow Area	Pipe Wt. per L.F.	Moment of Inertia	Section Modulus	Radius of Gyration
in.	in.	in.	in ²	lb	in ⁴	in ³	in.
0.5	0.147	0.546	0.234	1.09	0.020	0.048	0.250
0.75	0.154	0.742	0.432	1.47	0.045	0.085	0.321
1	0.179	0.957	0.719	2.17	0.106	0.161	0.407
1.25	0.191	1.278	1.282	3.00	0.242	0.291	0.524
1.5	0.200	1.500	1.766	3.63	0.391	0.412	0.605
		20 -				X: ::	8
2	0.218	1.939	2.951	5.02	0.868	0.731	0.766
2.5	0.276	2.323	4.236	7.66	1.920	1.340	0.924
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6	0.432	5.761	26.053	28.57	40.500	12.200	2.190
8	0.500	7.625	45.640	43.39	106.000	24.500	2.880
10	0.593	9.564	71.804	54.74	212.000	39.400	3.630
12	0.687	11.376	101.590	65.42	362.000	56.700	4.330

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3	0.600	2.300	4.153	18.58	5.990	3.420	1.050
4	0.674	3.152	7.799	27.54	15.300	6.790	1.370
5	0.750	4.063	12.959	38.55	33.600	12.100	1.720
6	0.864	4.897	18.825	53.16	66.300	20.000	2.060
8	0.875	6.875	37.104	72.42	162.000	37.600	2.760

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Bending Moment, Vertical Shear, and Deflection of Beams of Uniform Cross Section, **Under Various Conditions of Loading**

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P =concentrated loads, in lb

 R_1, R_2 = reactions, in lb

w = uniform load per unit of length, in lb per in.

W = total uniform load on beam, in lb

l =length of beam, in in.

x =distance from support to any section, in in.

E =modulus of elasticity, in psi

I =moment of inertia, in in⁴

 $V_{\rm r}$ = vertical shear at any section, in lb

V = maximum vertical shear, in lb

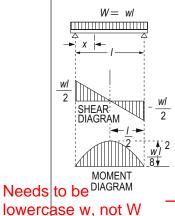
 M_{ν} = bending moment at any section, in lb-in.

M =maximum bending moment, in lb-in.

y = maximum deflection, in in.

k =fraction of l to P

Simple Beam: Uniform Load



$$R_1 = R_2 = \frac{wl}{2}$$

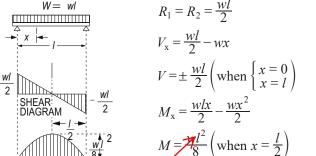
$$V_x = \frac{wl}{2} - wx$$

$$V = \pm \frac{wl}{2} \left(\text{when } \left\{ \begin{array}{l} x = 0 \\ x = l \end{array} \right)$$

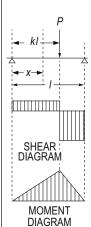
$$M_x = \frac{wlx}{2} - \frac{wx^2}{2}$$

$$M = \frac{l^2}{8} \left(\text{when } x = \frac{l}{2} \right)$$

 $y = \frac{5Wl^4}{384EI}$ (at center of span)

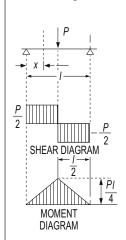


Simple Beam: Concentrated Load at Any Point



 $R_1 = P(1-k)$ $R_2 = Pk$ $V_x = R_1$ (when x < kl) $= R_2$ (when x > kl) V = P(1-k) (when k < 0.5) =-Pk (when k > 0.5) $M_x = Px(1-k)$ (when x < kl) = Pk(l-x) (when x > kl) M = Pkl(1-k) (at point of load) $y = \frac{Pl^3}{3EI} (1 - k) \times \left(\frac{2}{3}k - \frac{1}{3}k^2\right)^{\frac{3}{2}}$ $\left(\text{at } x = l\sqrt{\frac{2}{3}k - \frac{1}{3}k^2}\right)$

Simple Beam: Concentrated Load at Center



$$R_1 = R_2 = \frac{P}{2}$$

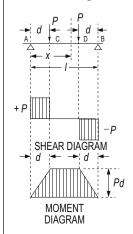
$$V_x = V = \pm \frac{P}{2}$$

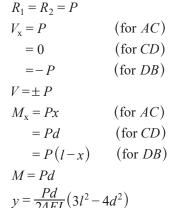
$$M_x = \frac{Px}{2}$$

$$M = \frac{Pl}{4} \left(\text{when } x = \frac{l}{2} \right)$$

$$y = \frac{Pl^3}{48EI} \text{ (at center of span)}$$

Simple Beam: Two Equal Concentrated Loads at **Equal Distances from Supports**





 $y = \frac{Pd}{24EI}(3l^2 - 4d^2)$ (at center of span)

Bending Moment, Vertical Shear, and Deflection of Beams of Uniform Cross Section, Under Various Conditions of Loading

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P =concentrated loads, in lb

 R_1, R_2 = reactions, in lb

w = uniform load per unit of length, in lb per in.

W = total uniform load on beam, in lb

l =length of beam, in in.

x = distance from support to any section, in in.

E =modulus of elasticity, in psi

I =moment of inertia, in in⁴

 $V_{\rm x}$ = vertical shear at any section, in lb

V = maximum vertical shear, in lb

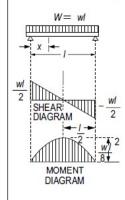
 $M_{\rm v}$ = bending moment at any section, in lb-in.

M = maximum bending moment, in lb-in.

y =maximum deflection, in in.

k =fraction of l to P

Simple Beam: Uniform Load



$$R_1 = R_2 = \frac{wl}{2}$$

$$V_{\rm x} = \frac{wl}{2} - wx$$

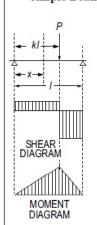
$$V = \pm \frac{wl}{2} \left(\text{when } \begin{cases} x = 0 \\ x = l \end{cases} \right)$$

$$M_{\rm x} = \frac{wlx}{2} - \frac{wx^2}{2}$$

$$M = \frac{wl^2}{8} \left(\text{when } x = \frac{l}{2} \right)$$

$$y = \frac{5wI^4}{384EI}$$
 (at center of span)

Simple Beam: Concentrated Load at Any Point $R_1 = P(1-k)$



 $R_2 = Pk$ $V_x = R_1 \quad \text{(when } x < kl)$

 $=R_2$ (when x > kl)

V = P(1-k) (when k < 0.5)

=-Pk (when k > 0.5)

 $M_x = Px(1-k)$ (when x < kl)

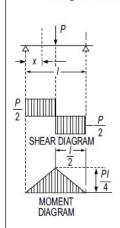
= Pk(l-x) (when x > kl)

M = Pkl(1-k) (at point of load)

$$y = \frac{Pl^3}{3EI} (1 - k) \times \left(\frac{2}{3}k - \frac{1}{3}k^2\right)^{\frac{3}{2}}$$

$$\left(\text{at } x = l\sqrt{\frac{2}{3}k - \frac{1}{3}k^2}\right)$$

Simple Beam: Concentrated Load at Center



$$R_1 = R_2 = \frac{P}{2}$$

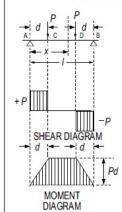
$$V_{\rm x} = V = \pm \frac{P}{2}$$

$$M_{\rm x} = \frac{Px}{2}$$

$$M = \frac{Pl}{4} \left(\text{when } x = \frac{l}{2} \right)$$

$$y = \frac{Pl^3}{48EI}$$
 (at center of span)

Simple Beam: Two Equal Concentrated Loads at Equal Distances from Supports



- $R_1 = R_2 = P$ $V_x = P \qquad \text{(for } AC\text{)}$ $= 0 \qquad \text{(for } CD\text{)}$
 - =-P (for DB)

 $V = \pm P$

 $M_{\rm x} = Px$ (for AC)

= Pd (for CD)

=P(I-x) (for DB)

M = Pd

 $y = \frac{Pd}{24EI} \left(3I^2 - 4d^2 \right)$

(at center of span)

Bending Moment, Vertical Shear, and Deflection of Beams of Uniform Cross Section, Under Various Conditions of Loading (cont'd)

P =concentrated loads, in lb

 R_1, R_2 = reactions, in lb

w = uniform load per unit of length, in lb per in.

W = total uniform load on beam, in lb

l = length of beam, in in.

x = distance from support to any section, in in.

E =modulus of elasticity, in psi

I = moment of inertia, in in⁴

 $V_{\rm x}$ = vertical shear at any section, in lb

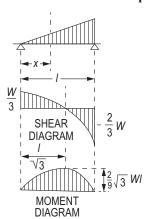
V = maximum vertical shear, in lb

 $M_{\rm v}$ = bending moment at any section, in lb-in.

M = maximum bending moment, in lb-in.

y = maximum deflection, in in.

Simple Beam: Load Increasing Uniformly from One Support to the Other



$$R_1 = \frac{W}{3}; R_2 = \frac{2}{3}W$$

$$V_{\rm x} = W\left(\frac{1}{3} - \frac{x^2}{l^2}\right)$$

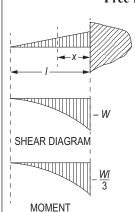
$$V = -\frac{2}{3}W \text{ (when } x = l)$$

$$M_{\rm x} = \frac{Wx}{3} \left(1 - \frac{x^2}{l^2} \right)$$

$$M = \frac{2}{9\sqrt{3}} Wl \left(\text{when } x = \frac{l}{\sqrt{3}} \right)$$

$$y = \frac{0.01304}{EI} W l^3$$

Cantilever Beam: Load Increasing Uniformly from Free End to Support



DIAGRAM

$$R = W$$

$$V_{x} = -W \frac{(l-x)^{2}}{l^{2}}$$

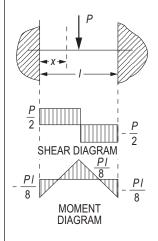
$$V = -W$$
 (when $x = 0$)

$$M_{\rm x} = -\frac{W}{3} \frac{\left(l - x\right)^3}{l^2}$$

$$M = -\frac{Wl}{3}$$
 (when $x = 0$)

$$y = \frac{Wl^3}{15EI}$$

Fixed Beam: Concentrated Load at Center of Span



$$R_1 = R_2 = \frac{P}{2}$$

$$V_{\rm x} = V = \pm \frac{P}{2}$$

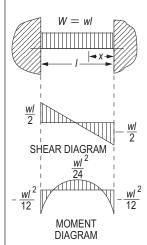
$$M_{\rm x} = P\left(\frac{x}{2} - \frac{l}{8}\right)$$

$$M_{\rm x} = -\frac{Pl}{8} \left(\text{when } \left\{ \begin{array}{l} x = 0 \\ x = l \end{array} \right. \right)$$

$$M = +\frac{Pl}{8}$$
 (at center of span)

$$y = \frac{WI^3}{192EI}$$

Fixed Beam: Uniform Load



$$R_1 = R_2 = \frac{wl}{2} = \frac{W}{2}$$

$$V_{\rm x} = \frac{wl}{2} - wx$$

$$V = \pm \frac{wl}{2}$$
 (at ends)

$$M_{\rm x} = -\frac{wl^2}{2} \left(\frac{1}{6} - \frac{x}{l} + \frac{x^2}{l^2} \right)$$

$$M = -\frac{1}{12}wl^2 \left(\text{when } \left\{ \begin{array}{l} x = 0 \\ x = l \end{array} \right. \right)$$

$$M = \frac{wl^2}{24} \left(\text{when } x = \frac{l}{2} \right)$$

$$y = \frac{Wl^3}{384EI}$$

Should be a P, not a W

Bending Moment, Vertical Shear, and Deflection of Beams of Uniform Cross Section, Under Various Conditions of Loading (cont'd)

P =concentrated loads, in lb

 R_1, R_2 = reactions, in lb

w = uniform load per unit of length, in lb per in.

W = total uniform load on beam, in lb

l =length of beam, in in.

x = distance from support to any section, in in.

E =modulus of elasticity, in psi

I =moment of inertia, in in⁴

 $V_{\rm x}$ = vertical shear at any section, in lb

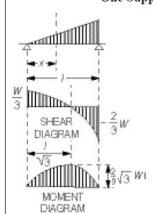
V = maximum vertical shear, in lb

 $M_{\rm x}$ = bending moment at any section, in lb-in.

M = maximum bending moment, in lb-in.

y =maximum deflection, in in.

Simple Beam: Load Increasing Uniformly from One Support to the Other



$$R_1 = \frac{W}{3}$$
; $R_2 = \frac{2}{3}W$

$$V_{\rm x} = W\left(\frac{1}{3} - \frac{x^2}{l^2}\right)$$

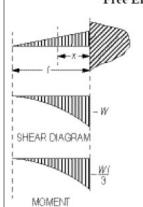
$$V = -\frac{2}{3}W$$
 (when $x = l$)

$$M_{\rm x} = \frac{Wx}{3} \left(1 - \frac{x^2}{l^2} \right)$$

$$M = \frac{2}{9\sqrt{3}} WI \left(\text{when } x = \frac{l}{\sqrt{3}} \right)$$

$$y = \frac{0.01304}{EI} W l^3$$

Cantilever Beam: Load Increasing Uniformly from Free End to Support



DIAGRAM

$$R = W$$

$$V_{x} = -W \frac{(l-x)^{2}}{l^{2}}$$

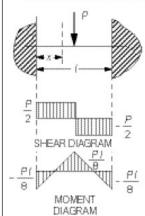
$$V = -W \text{ (when } x = 0)$$

$$M_{\rm x} = -\frac{W}{3} \frac{(l-x)^3}{l^2}$$

$$M = -\frac{WI}{3}$$
 (when $x = 0$)

$$y = \frac{Wl^3}{15EI}$$

Fixed Beam: Concentrated Load at Center of Span



$$R_1 = R_2 = \frac{P}{2}$$

$$V_{\rm x} = V = \pm \frac{P}{2}$$

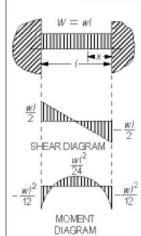
$$M_{\rm x} = P\left(\frac{x}{2} - \frac{l}{8}\right)$$

$$M_{\rm x} = -\frac{Pl}{8} \left(\text{when } \begin{cases} x = 0 \\ x = l \end{cases} \right)$$

$$M = +\frac{Pl}{8}$$
 (at center of span)

$$y = \frac{Pl^3}{192EI}$$

Fixed Beam: Uniform Load



$$R_1 = R_2 = \frac{wl}{2} = \frac{W}{2}$$

$$V_{\rm x} = \frac{wl}{2} - wx$$

$$V = \pm \frac{wl}{2}$$
 (at ends)

$$M_{\rm x} = -\frac{wl^2}{2} \left(\frac{1}{6} - \frac{x}{l} + \frac{x^2}{l^2} \right)$$

$$M = -\frac{1}{12}wl^2 \left(\text{when } \begin{cases} x = 0 \\ x = l \end{cases} \right)$$

$$M = \frac{wl^2}{24} \left(\text{when } x = \frac{l}{2} \right)$$

$$y = \frac{Wl^3}{384EI}$$

New Section Added:

1.22 Sound Decibel Addition: Decibels are logarithmic ratios, two decibel values cannot be added directly together to obtain their sum. Instead, the total level can be calculated by determining the difference between the two levels and adding a correction to the larger of the two using the equation below.

$$\Sigma (L_1 + L_2) = L_1 + 10 \log_{10}(1 + 10 - \frac{\Delta L}{10})$$

where

 L_1 = higher decibel level

 $L_2 =$ lower decibel level $\Delta L = L_1 - L_2$

$$\Delta L = L_1 - L_2$$

Moment to Change Trim One CM

SI units:

$$MTcm \frac{\Delta BM_L}{100L} = \frac{0.01025 I_L}{L}$$
 (salt water)

$$MTcm \frac{\Delta BM_L}{100L} = \frac{I_L}{L}$$
 (fresh water)

Updated:

Moment to Change Trim One CM

SI units:

$$MTcm \frac{\Delta BM_L}{100L} = \frac{0.01025 I_L}{L}$$
 (salt water)

$$MTcm \frac{\Delta BM_L}{100L} = \frac{I_L}{L}$$
 (fresh water)

6.3.5 Darcy-Weisbach Equation

$$h_L = f \, \frac{L}{D} \, \frac{v^2}{2g}$$

where

should be hL, not h_f

 h_f head loss

f = friction factor, dimensionless

D = internal diameter of the pipe

L = length over which the head loss occurs

v = fluid velocity

g = acceleration of gravity

Updated:

$$h_L = f \, \frac{L}{D} \, \frac{v^2}{2g}$$

where

hL = head loss

f = friction factor, dimensionless

D = internal diameter of the pipe

L =length over which the head loss occurs

v = fluid velocity

g = acceleration of gravity

6.3.15 Pilot Tubes

Updated:

6.3.15 Pitot Tubes

Added Section 6.3.20 Hydraulic Cylinders

6.3.20 Hydraulic Cylinders

Hydraulic cylinder systems with 2 or more cylinders obey the following laws of similitude that maintain constant pressure and volume (for incompressible fluid):

$$F_1 * A_1 = F_2 * A_2$$

Where F = force input/output by a hydraulic cylinder and A = cross-sectional area of the hydraulic cylinder.

$$S_1 * A_1 = S_2 * A_2$$

Where S = stroke of the hydraulic cylinder.

Internal Combustion Engines

The mean effective pressure equals net work divided by volumetric displacement. Horsepower is derived from

$$hp = (MEP) \frac{Lan}{K}$$

where

MEP = mean effective pressure, in $\frac{lb}{in^2}$ or kPa

L = stroke, in ft or m

 $a = \text{total piston area, in in}^2 \text{ or m}^2$

n = number of cycles completed per min

K = 33,000 for I-P units or 0.4566 for SI units

-Should be 0.04472

 $r_{\rm v}$ = compression ratio = $\frac{V_1}{V_2} = \frac{V_4}{V_3}$

$$\frac{T_2}{T_1} = \left(\frac{V_1}{V_2}\right)^{k-1} \qquad \frac{T_3}{T_4} = \left(\frac{V_4}{V_3}\right)^{k-1}$$

$$\frac{P_2}{P_1} = \left(\frac{V_1}{V_2}\right)^k \qquad \qquad \frac{P_3}{P_4} = \left(\frac{V_4}{V_3}\right)^k$$

Updated:

Internal Combustion Engines

The mean effective pressure equals net work divided by volumetric displacement. Horsepower is derived from

$$hp = (MEP) \frac{Lan}{K}$$

where

MEP = mean effective pressure, in $\frac{lb}{in^2}$ or kPa

L = stroke, in ft or m

 $a = \text{total piston area, in in}^2 \text{ or m}^2$

n = number of cycles completed per min

K = 33,000 for I-P units or 0.04472 for SI units

 $r_{\rm v} = {
m compression \ ratio} = \frac{V_1}{V_2} = \frac{V_4}{V_3}$

$$\frac{T_2}{T_1} = \left(\frac{V_1}{V_2}\right)^{k-1} \qquad \qquad \frac{T_3}{T_4} = \left(\frac{V_4}{V_3}\right)^{k-1}$$

$$\frac{P_2}{P_1} = \left(\frac{V_1}{V_2}\right)^k \qquad \qquad \frac{P_3}{P_4} = \left(\frac{V_4}{V_3}\right)^k$$

Added Bearing Force Equation

7.11 Bearing Force

Bearing Force for rotating shaft with weight spread equally between bearings:

$$F_B = 0.5W_R(1+(r\omega^2)/g)$$

where

 $F_B = Bearing Force$

W_R = Rotating Shaft Weight

r = off-center distance from rotational axis to center of mass

 ω = rate of rotation (rad/sec)

g = acceleration due to gravity

Added a new equation under Gear Pitch Original:

7.13 Gears

7.13.1 Gear Pitch

$$P = \frac{N}{d}$$

$$F_c = \frac{T}{P}$$

where

P = Diametral Pitch

N = Number of teeth

d = Pitch diameter

F_c = Gear contact or tangential force

T = Torque

Updated:

7.14 Gears

7.14.1 Gear Pitch

$$P = \frac{N}{d}$$

$$F_c = \frac{T}{P}$$

P = Diametral Pitch

N = Number of teeth

d = Pitch diameter

 F_c = Gear contact or tangential force

T = Torque

Wt = 126050*(HP/(RPMp x d))

Where

Wt = tooth loading

RPMp = pinion RPM

d = pitch diameter of the pinion

8.4 Refrigeration

8.4.1 Compression Refrigeration Cycles

Refer to Chapter 4, Thermodynamics, for additional information on compression refrigeration cycles.

Should be Chapter 7

Updated:

8.4 Refrigeration

8.4.1 Compression Refrigeration Cycles

Refer to Chapter 7, Thermodynamics, for additional information on compression refrigeration cycles.